

Two-Phase Continuum Models for Geophysical Particle-Fluid Flows

Time-dependent measurements for incipient bed load discharge on shallow open channel flows

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supervisors :

Pascal DUPONT

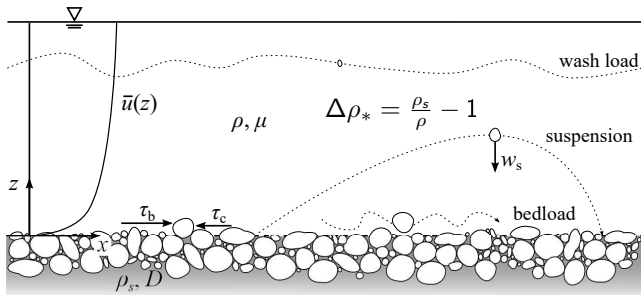
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General framework



$q_0 ; u_0 ; h_0 ;$
 $\rightarrow \tau_b ; u_* ;$
 $u_* > u_{*c} :$
 Transport !

Shields number

$$Sh = u_*^2 / (\Delta\rho_* g D) ;$$

Transport

$$\rightarrow q_* = A(Sh - Sh_c)^B .$$

(Shields, 1936; Paphitis, 2001; Beheshti and Ataie-Ashtiani, 2008)

(Meyer-Peter and Müller, 1948; Wong and Parker, 2006)

Dimensionless analysis

Reynolds : $Re = 4 \frac{u_0 h_0}{\nu}$ flow dynamics ;

Froude : $Fr = \frac{u_0}{\sqrt{gh_0 \cos \theta}}$ flow hydraulic regimen ;

Shields : $Sh = \frac{u_*^2}{\Delta \rho_* g D}$ particles initiation of motion ;

Particle Reynolds : $Re_p = \frac{u_0 D}{\nu}$ particle-flow interaction ;

Stokes : $St = \frac{T_p}{T_f}$ particles-flow interaction ;

Particle Froude : $Fr_p = \frac{u_0}{\sqrt{gD}}$ flow capacity of transport ;

Rouse : $\frac{w_s}{\kappa u_*}$; $\frac{w_s}{u_*}$; $\frac{u_*}{w_s}$ dominant mode of sediment transport ;
(suspension or movability)

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Reynolds : $Re = 4 \frac{u_0 h_0}{\nu}$

Froude : $Fr = \frac{u_0}{\sqrt{gh_0 \cos \theta}}$

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Particle Reynolds : $Re_p = \frac{u_0 D}{\nu}$

Stokes : $St = \frac{T_p}{T_f}$

Particle Froude : $Fr_p = \frac{u_0}{\sqrt{gD}}$

Rouse : $\frac{w_s}{\kappa u_*}; \frac{w_s}{u_*}; \frac{u_*}{w_s}$
(suspension or movability)

flow dynamics ;

flow hydraulic regimen ;

particles initiation of motion ;

particle-flow interaction ;

particles-flow interaction ;

flow capacity of transport ;

dominant mode of sediment transport ;

Non-stationary transport - Turbulence

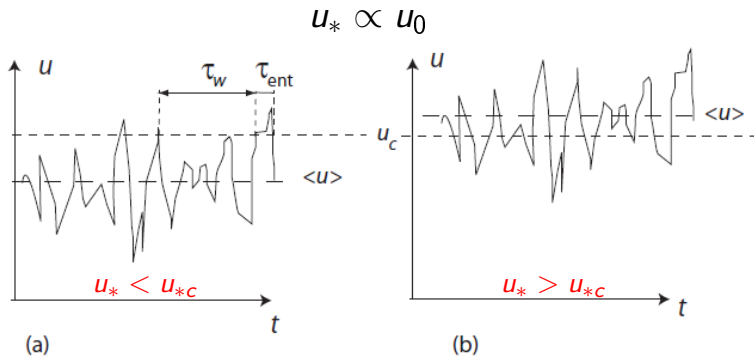


Figure 1 – Fluctuations of the instantaneous velocity around the mean value. Particle entrainment occurs sporadically. (Ancy et al., 2008).

Non-stationary transport - Grain-size distribution

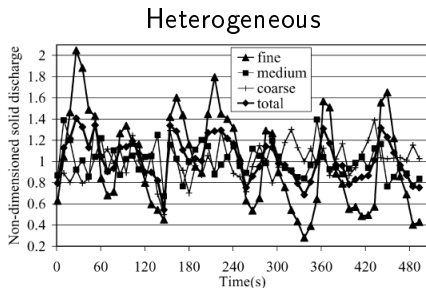


Fig. 17. Solid discharges of fine, medium, coarse materials and total (run 4)

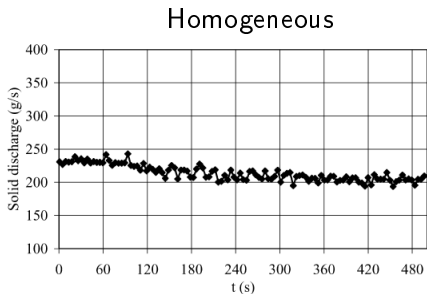


Fig. 18. Time series of solid discharge for the uniform material (run 13)

Figure 2 – Solid discharges of materials with different granulometry (Frey et al., 2003).

Non-stationary transport - Flow instabilities

Free surface instabilities

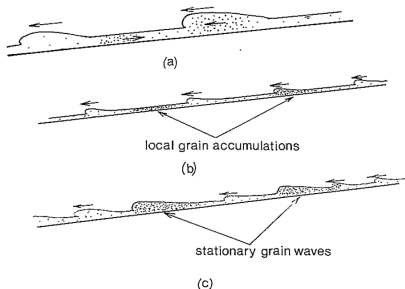


FIG. 8—(a) Roll waves with grains in flow (b) Development of local stationary grain accumulations (c) Development of stationary grain waves.

Figure 3 – Evolution of roll waves and sediment transport (Davies, 1990).

& Bedforms

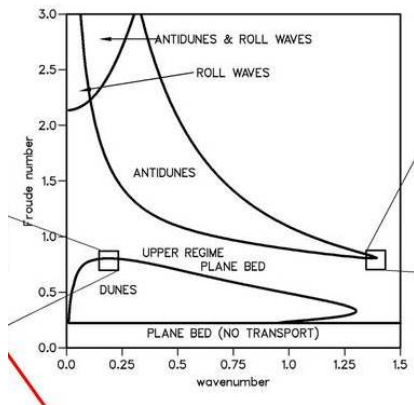


Figure 4 – Comparison of stability lines for roll waves and bedforms (Colombini and Stocchino, 2005).

Main objective

Non-stationary sediment transport

Experimentally study time-dependent effects on sediment transport for runoff flows.

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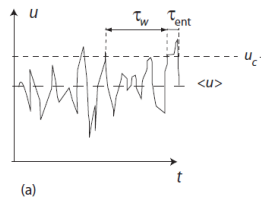
Close to threshold cases :

Shields number

$$Sh < 2.5Sh_c ;$$

(Recking et al., 2009)

Turbulence effects :



Experimental project

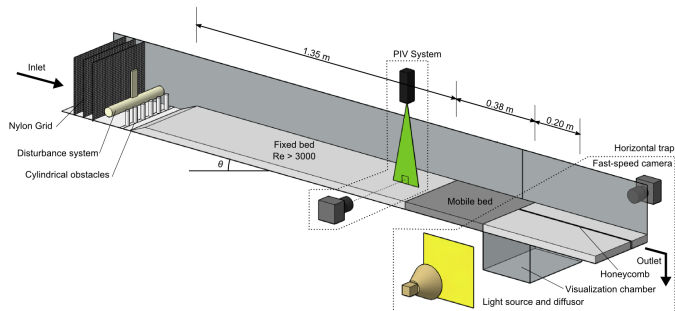


Figure 5 – Sketch of experimental setup.

- ▶ shallow water flow $h_0/l \ll 1$;
- ▶ PIV method for local flow dynamics measurements ;
- ▶ contact needle for global measurements ;
- ▶ combined shadowgraph and PTV methods for local sediment transport measurements ;

Test sand

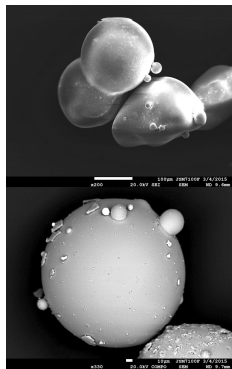
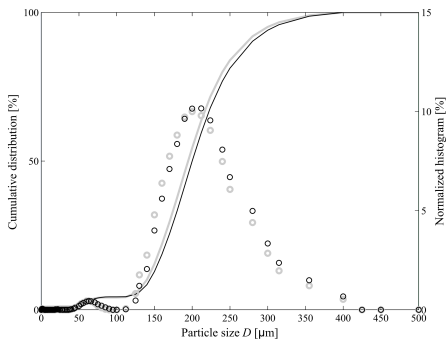


Figure 6 – Histogram for particles used in experiments and MEV images.

- ▶ non-cohesive;
- ▶ regular shape;
- ▶ $0.15 < D < 0.25$ mm;
- ▶ $Sh_c = 0.062$;
- ▶ favorable to bedload (given the exp. limitations, $w_s \approx 2\text{cm/s}$);

Dimensionless parameters

Dimensionless	Range	Interpretation
Re	$3000 < \text{Re} < 4000$	Turbulent (close to transition);
Fr	$1.1 < \text{Fr} < 1.5$	Torrential flows
Sh	$0.068 < \text{Sh} < 0.165$	particles initiation of motion;
Rouse :	$2.4 < \frac{w_s}{u_*} < 3.6$	bed-load transport ;

$$\text{Sh}_c = 0.062 \text{ (Paphitis, 2001)}$$

Methodology debrief

For flow dynamics

- ▶ determination of friction velocity u_* based on theoretical turbulent profile; assumption $u_*(t) = F\{\langle u \rangle (y_0, t)\}$; correction of results based on global values (contact needle);
- ▶ Obtaining of time-dependent friction velocity $u_*(t)$.

For sediment transport

- ▶ Image acquisition from fast-recording camera; images processing on Matlab; PTV algorithm; correction of particles size/velocities; time dependent discharge computation; comparison to global values from mean weighted discharge;
- ▶ Obtaining of time-variable transport rate $q(t)$.

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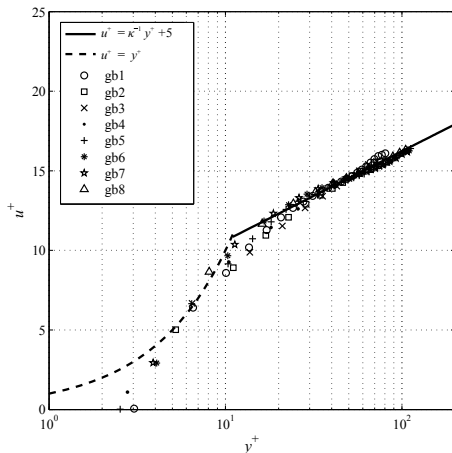
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Statistically...

$$q(t) \stackrel{?}{=} f\{u_*(t)\}$$

Friction velocity obtaining



In wall coordinates :

$$u^+ = u(y)/u_*$$

and

$$y^+ = u_* y / \nu$$

For viscous sublayer :

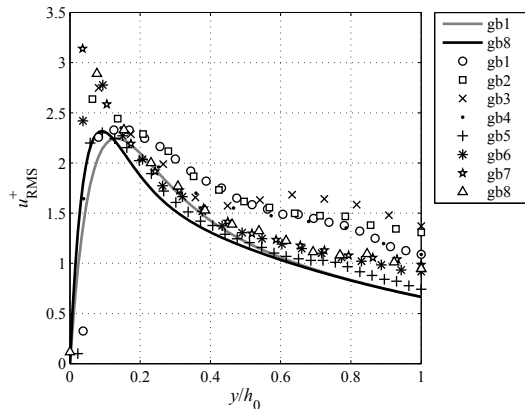
$$u^+ = y^+ ;$$

For log region :

$$u^+ = \kappa^{-1} \log y^+ + 5 ;$$

Figure 7 – Turbulent characteristics of average profile of mean flow velocity. Lines indicate theoretical values : solid line is $u^+ = \kappa^{-1} \log y^+ + 5$ for log region ; dashed line is $u^+ = y^+$ for viscous layer.

Turbulent intensities - u_{RMS}^+



(Antonia and Krogstad, 2001)

$$u_{RMS}^+ = A \exp\left(-\frac{y^+}{Re_*}\right) \left[1 - \exp\left(-\frac{y^+}{B}\right)\right] + C y^+ \exp\left(-\frac{y^+}{B}\right)$$

$A = 2$, $B = 8$, and $C = 0.34$;
 $Re_* = u_{*PIV} h_0 / \nu$ is the Reynolds number using friction velocity as reference.

Figure 8 – Turbulent intensities for runs gb1 to gb8. Lines indicate computed values following empirical results from Antonia and Krogstad (2001). Longitudinal turbulent intensities u_{RMS}^+ ; dark line represent run computed value for gb1, and gray line, for gb8.

Friction velocity $u_*(t)$

Bottom shear stress :

$$\tau_b \sim \rho u_*^2 \sim \rho g h_0 \sin \theta$$

For our experiments :

- ▶ $u_* \sim 0.01 \text{ ms}^{-1}$;
 $\rightarrow \tau_b \sim (10^3)(10^{-2})^2 \sim 0.1 \text{ Pa}$;

- ▶ $h_0 \sim 0.01 \text{ m}$;
 $\rightarrow \tau_b \sim (10^3)(10^1)(10^{-2})(10^{-2}) \sim 1 \text{ Pa}$;

The precision required to
measure fluctuations
would be $\sim 0.01 \text{ Pa}$

!!!

(Detert et al., 2010; Amir
et al., 2014)

So, an indirect measure is pursued...

Friction velocity correlation

$\tau_b(t)$?

- ▶ $u_*^2 \propto \tau_b \propto u_0^2$
- ▶ $u(y_0) \propto u_0 \rightarrow u_* \propto u(y_0)$

Dimensionally and statistically we can assume that :

- ▶ $\bar{u}_*^2(t) \propto \bar{\tau}_b(t) \propto \bar{u}^2(y_0, t)$
- ▶ so there is a function F that :
 $\bar{u}_*(t) = F\{\bar{u}(y_0, t)\}$

(Ould Ahmedou et al., 2007)

- ▶ the hypothesis : F is also valid for instantaneous variables, so that :
 $u_*(t) = F\{u(y_0, t)\}$

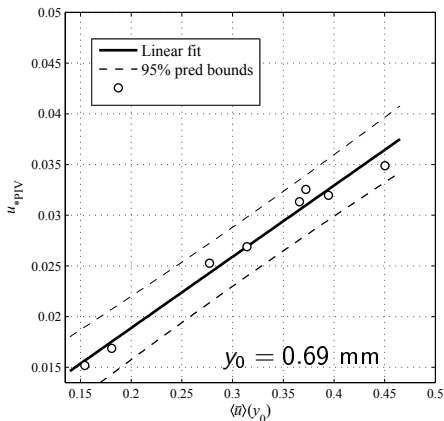


Figure 9 – Correlation between $\langle \bar{u} \rangle(y_0)$ and u_{*PIV} . First-order polynomial approximation and 95% confidence boundaries.

Friction velocity correlation

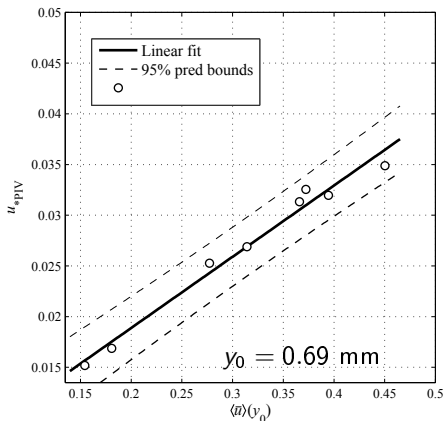
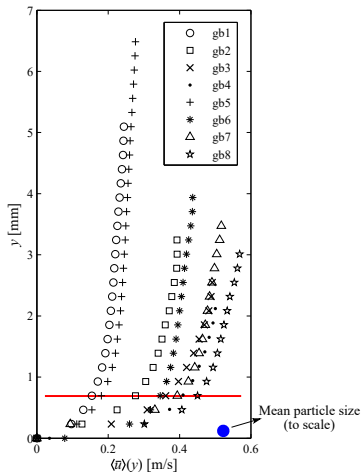
 $\tau_b(t) ?$ 

Figure 9 – Correlation between $\langle \bar{u} \rangle(y_0)$ and u_{*PIV} . First-order polynomial approximation and 95% confidence boundaries.

Friction velocity signal

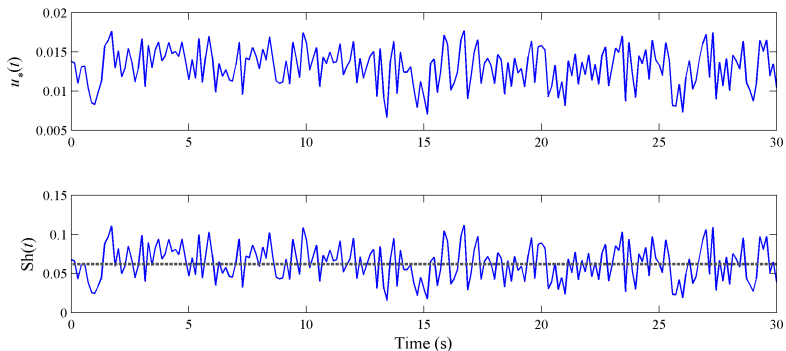
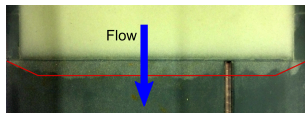
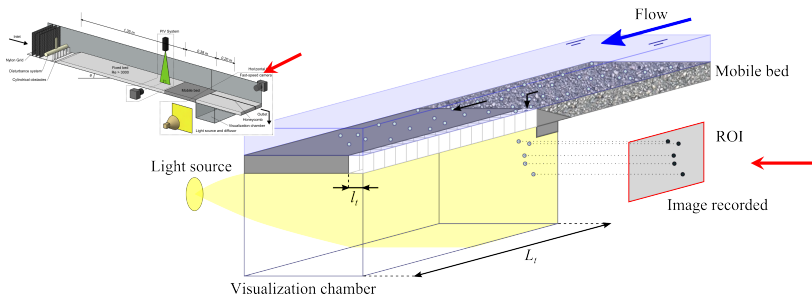
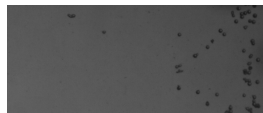


Figure 10 – Signals of $u_*(t)$ and $Sh(t)$ for a close to threshold experiment run. The dotted dark line represent Sh_c .

Experimental project

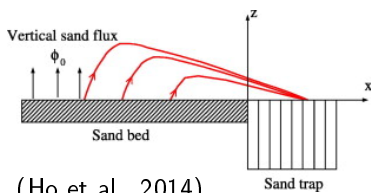
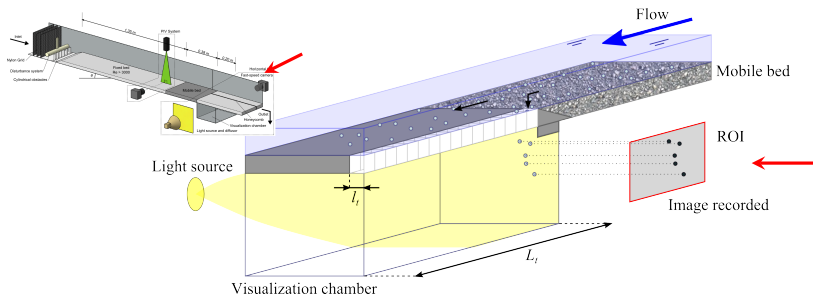


Top view of the trap.

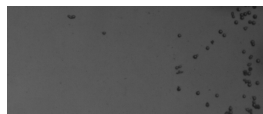


Side view, camera perspective.
(red arrow)

Experimental project



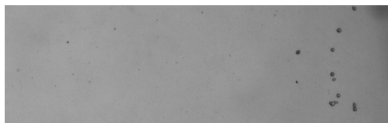
(Ho et al., 2014)



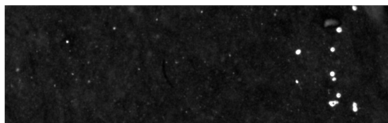
Side view, camera perspective.
(red arrow)

Particles identification method

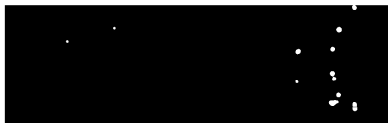
Original image



Linear histogram normalization

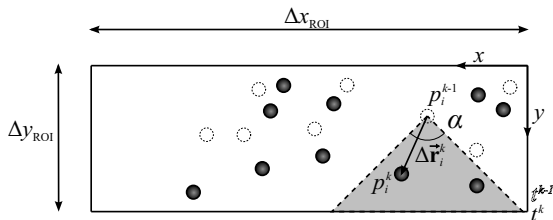


Binary Image



- ▶ Based on maximum and minimum images from series, each image gray level is adjusted;
- ▶ Canny filter is applied, using Otsu's threshold method, to identify particle edges;
- ▶ Morphological operations (closing, filling, opening, watershed) are performed.
(Frey et al., 2003)

Particle Tracking Velocimetry



Particle mass :

$$m_i^k = \frac{\pi \rho_s}{6} D_i^k{}^3$$

Particle displacement :

$$\Delta \vec{r}_i^k$$

Particle velocity :

$$\vec{v}_i^k = \frac{\Delta \vec{r}_i^k}{\Delta t}$$

Total mass at time t^k :

$$M(t^k) = \sum_i^{N_p} m_i^k ;$$

Time scale of particles permanence :

$$\Delta t_{\text{esc}}^k = \frac{\Delta y_{\text{ROI}}}{\bar{v}_{y_i}^k} ;$$

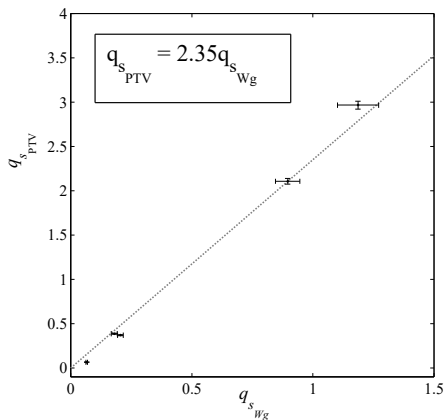
Time dependent discharge :

$$q(t^k) = \frac{M(t^k)}{\Delta t_{\text{esc}}^k} ,$$

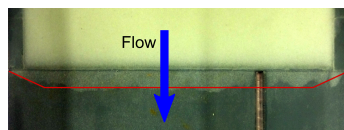
Mean : $\bar{v}_{y_i}^k = \frac{1}{N_p} \sum_i^{N_p} v_{y_i}^k ;$

Weighted-mean : $\bar{v}_{y_i}^k = \frac{1}{M(t^k)} \sum_i^{N_p} m_i^k v_{y_i}^k .$

Instantaneous discharge measurement



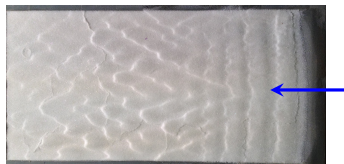
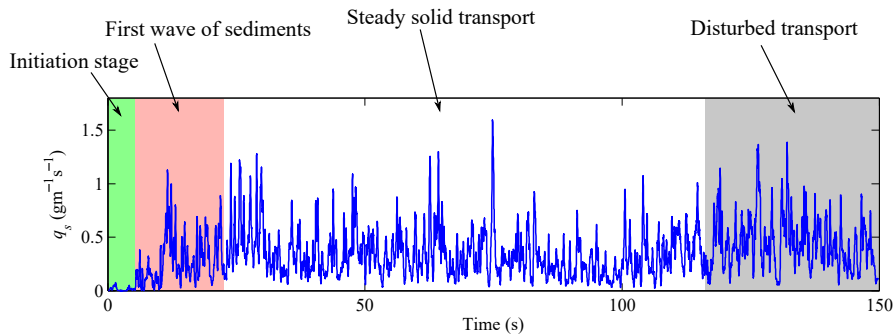
Measure	u_*	Sh	Fr	\bar{q}_{lW}	\bar{q}_{lPTV}
gb1-2	0.014	0.068	1.16	0.065	0.064
gb1-3	0.014	0.075	1.20	0.179	0.386
gb1-4	0.015	0.079	1.10	0.204	0.336
gb2-1	0.021	0.158	1.55	1.187	2.967
gb2-2	0.021	0.159	1.54	0.896	2.107



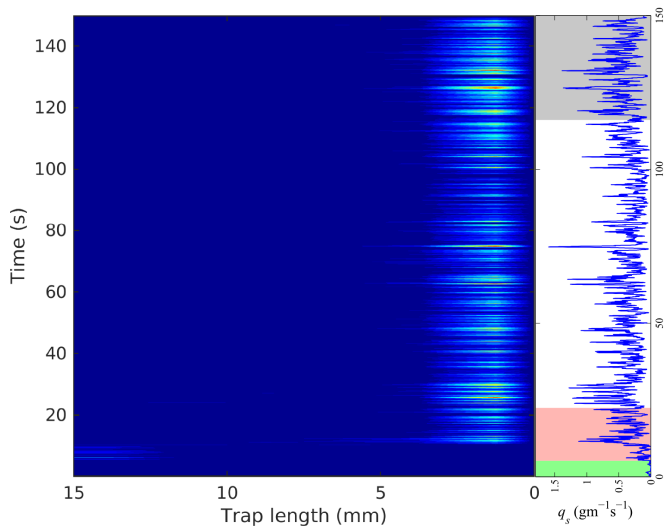
Top view of the trap.

Figure 11 – Correlation between sediment discharge computation based on images and weighted mass.

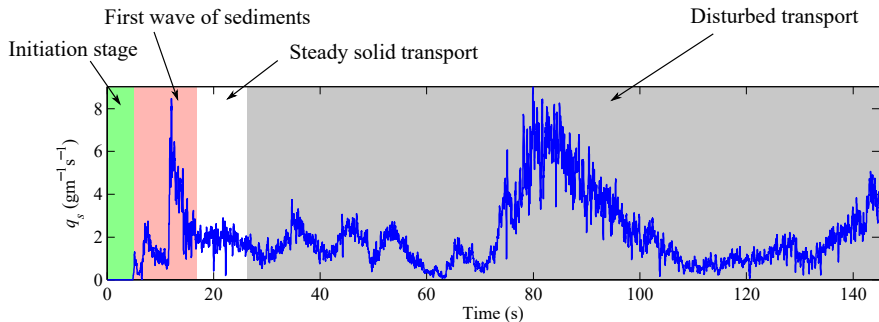
$$Sh = 0.079, Fr = 1.10$$



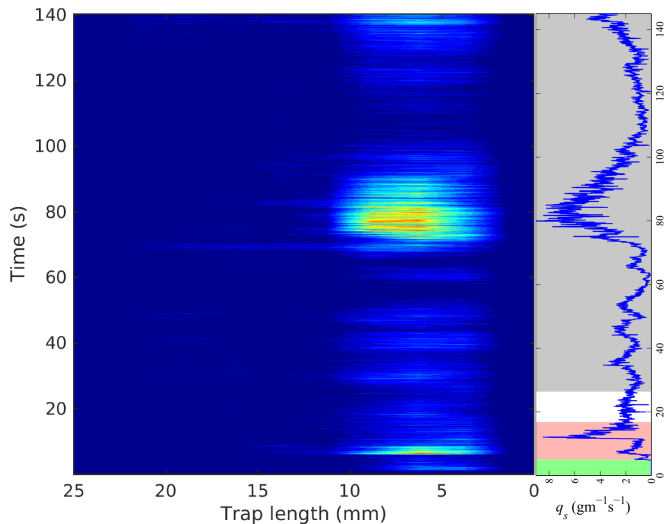
$$Sh = 0.079, Fr = 1.10$$



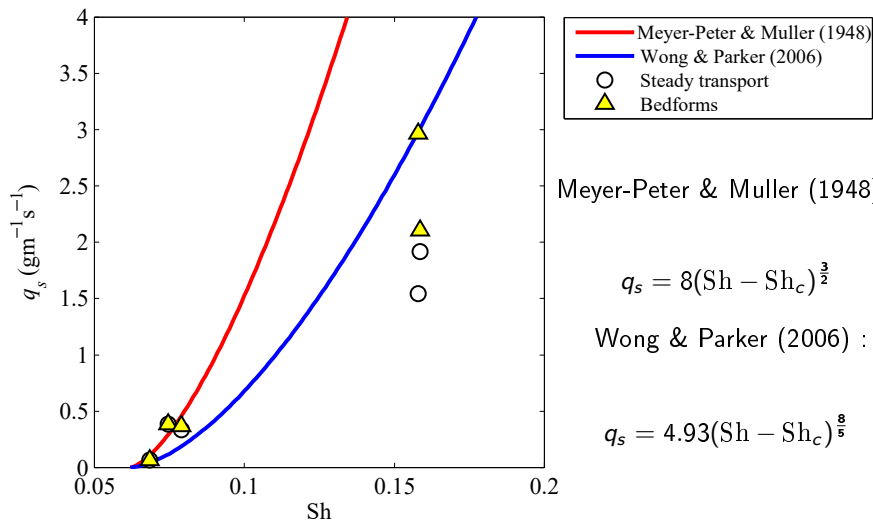
$$Sh = 0.159, Fr = 1.54$$



$$\text{Sh} = 0.159, \text{Fr} = 1.54$$



Comparison to classical formulas



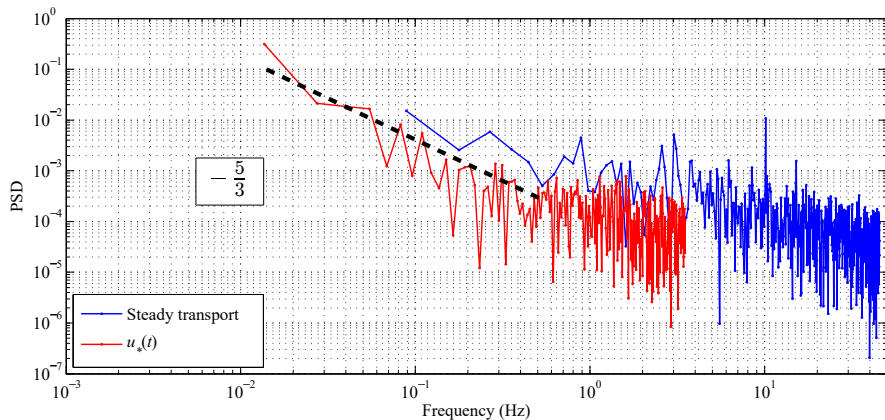
Meyer-Peter & Muller (1948) :

$$q_s = 8(\text{Sh} - \text{Sh}_c)^{\frac{3}{2}}$$

Wong & Parker (2006) :

$$q_s = 4.93(\text{Sh} - \text{Sh}_c)^{\frac{8}{5}}$$

PSD




Discussions


- ▶ q_{sRMS} and u_{*RMS} ;
- ▶ characteristic time scales ;
- ▶ additional effect : pulsating flows ;

Thank you !

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Mean flow velocity and turbulent intensities

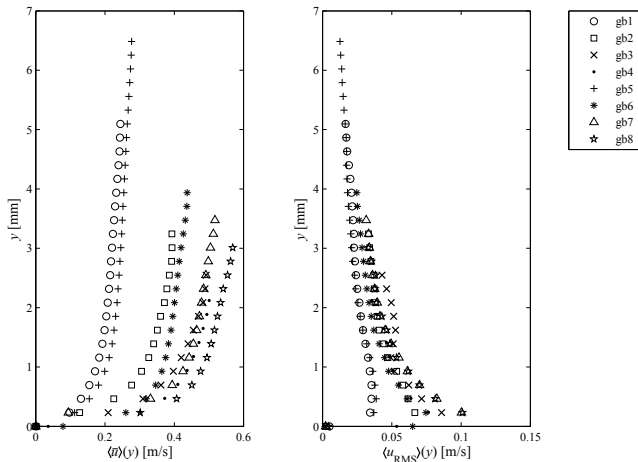
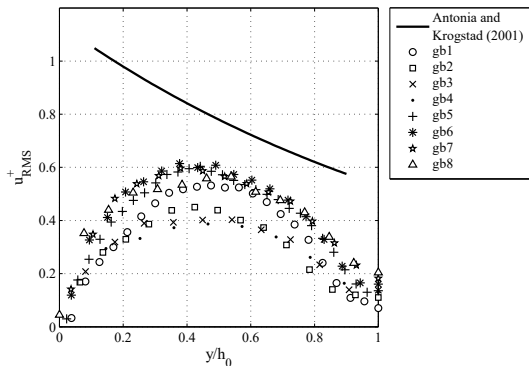


Figure 12 – Results for average profiles of mean flow velocity $\langle \bar{u} \rangle(y)$ and standard deviation $\langle u_{RMS} \rangle(y)$ for runs gb1 to gb8.

Turbulent intensities - v_{RMS}^+ 

(Antonia and Krogstad, 2001)

$$v_{RMS}^+ = 1.14 \exp\left(\frac{-0.76y}{h_0}\right)$$

Figure 13 – Turbulent intensities for runs gb1 to gb8. Lines indicate computed values following empirical results from Antonia and Krogstad (2001). Vertical turbulent intensities v_{RMS}^+ ; dark line represent computed values.

Friction velocity correction

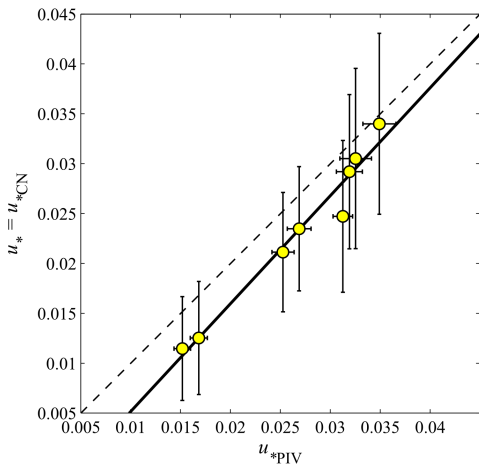


Figure 14 – Correlation between friction velocity computed through both methods u_{*CN} and u_{*PIV} . Solid line represent linear relation between both methods for friction velocity calculation. Dashed line represents equality $u_{*PIV} = u_{*CN}$.

U_*

$u_* = u_{*CN} = 1.08u_{*PIV} - 0.006$
with a correlation coefficient
 $R^2 = 0.96$.

Grain-size distribution

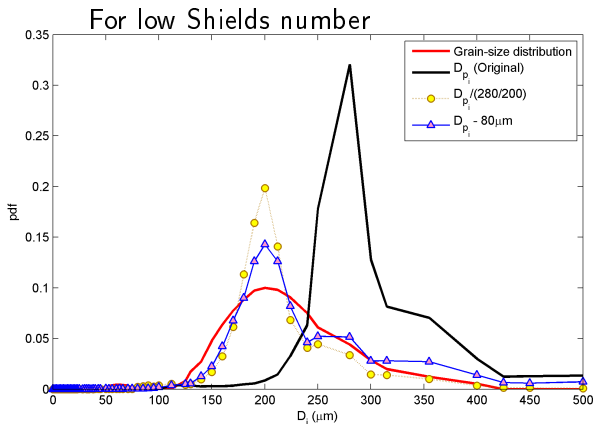


Figure 15 – Probability distribution function from captured images in comparison to calibrated grain-size distribution.

→ Correction of particles diameter.

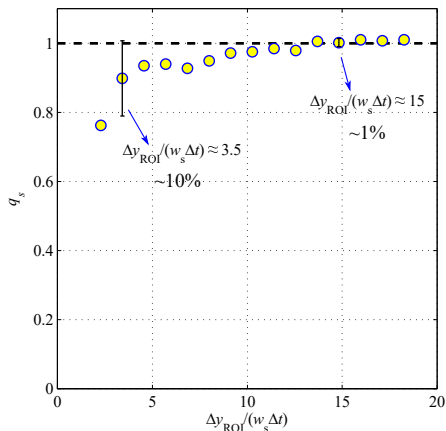
Δy_{ROI} influence

Figure 16 – Influence of Δy_{ROI} on $q_s(t)$.

Filtering of data, reducing discretization noise.

→ Imposes time window of signal acquisition : Δt_{esc} .

In order to be comparable to PIV signal : $\Delta y_{\text{ROI}} = 6.5\text{mm}$

($\Delta t_{\text{esc}} = 0.13 - 0.14\text{s}$)

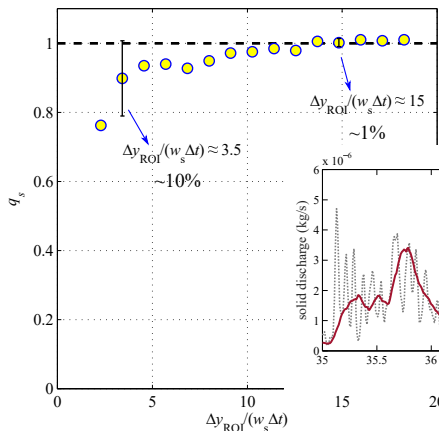
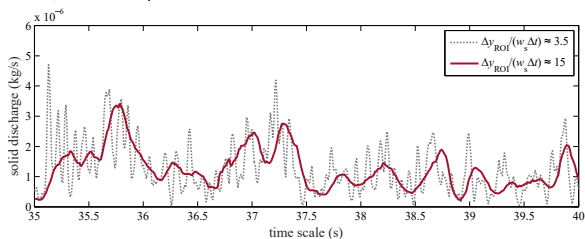
Δy_{ROI} influence

Figure 16 – Influence of Δy_{ROI} on $q_s(t)$.



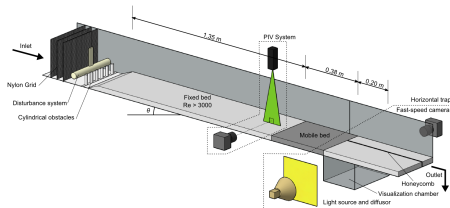
→ Imposes time window of signal acquisition : Δt_{esc} .

In order to be comparable to PIV signal : $\Delta y_{\text{ROI}} = 6.5\text{mm}$

($\Delta t_{\text{esc}} = 0.13 - 0.14\text{s}$)

Experimental run

1. arrange the experiment ;
2. partially block the outlet ;
3. set flow discharge and slope ;
4. start data acquisition (PIV and PTV) ;
5. release the outlet ;
6. after 1'30", data acquisition stops ;
7. block outlet ;



PSD

